

## TYPICAL DURA-HOLD II SINGLE CRIB WALL SAMPLE CALCULATIONS

Note: Calculations shown below are short form preliminary design analysis for the typical DURA-HOLD II single crib wall section shown on Plate No. DHII-6. Calculations are performed using the Coulomb Analysis Method for retaining wall design.

### Given:

H	=	10'-0"
i	=	26.56° (Backfill at 1:2 slope)
H <sub>a</sub>	=	10'-0" + 2'-6" = 12'-6"
h	=	3'-0"
b	=	6'-0"
γ <sub>wall</sub>	=	145 pcf
q	=	No surcharge
f	=	0.6
φ	=	30°
α	=	7.13° (Wall at natural 8:1 batter)
β	=	82.87°
δ	=	2/3 φ = 20°
γ <sub>soil</sub>	=	130 pcf (saturated)
γ <sub>stone</sub>	=	135 pcf (compacted)

### 1. Determine forces acting on wall:

#### a. Find K<sub>a</sub> and K<sub>p</sub>:

$$K_a = \left[ \frac{\operatorname{cosec} \beta \sin (\beta - \phi)}{\sqrt{\sin (\beta + \delta)} + \sqrt{\frac{\sin (\delta + \phi) \sin (\phi - i)}{\sin (\beta - i)}}} \right]^2$$

$$= \left[ \frac{\frac{\sin (82.87 - 30)}{\sin 82.87}}{\sqrt{\sin (82.87 + 20)} + \sqrt{\frac{\sin (20 + 30) \sin (30 - 26.56)}{\sin (82.87 - 26.56)}}} \right]^2$$

$$K_a = .43$$

$$K_p = \left[ \frac{\operatorname{cosec} \beta \sin (\beta + \phi)}{\sqrt{\sin (\beta - \delta)} \sqrt{\frac{\sin (\delta + \phi) \sin (\phi + i)}{\sin \beta - i}}} \right]^2$$

$$K_p = 1.72$$

b. Find  $P_a$ ,  $P_s$  and  $P_p$ :

$$\begin{aligned} P_a &= 1/2 K_a \gamma H_a^2 \\ &= (.5)(.43)(130)(12.5)^2 \\ &= 4367.19 \text{ lbs.} \end{aligned}$$

$$\begin{aligned} P_s &= K_a q H_a \\ &= .43 (0) (7.75) \\ &= 0 \text{ lbs. (No surcharge)} \end{aligned}$$

$$\begin{aligned} P_p &= 1/2 K_p \gamma h^2 \\ &= (.5)(1.72)(130)(3)^2 \\ &= 1006.2 \text{ lbs} \end{aligned}$$

*Note: Passive resistance will be used in this example. The contractor is directed to backfill footing and first two courses prior to continuing wall erection.*

2. Determine stability against sliding along the base:

a. Find  $F_s$ ,  $F_r$  and  $FS_s$

$$\begin{aligned} F_s &= (\Sigma \text{ Horizontal sliding forces}) \\ &= P_a \cos (\delta + \beta - 90) + P_s \cos (\delta + \beta - 90) \\ &= 4367.19(.97) + 0(.97) \\ &= 4236.17 \text{ lbs} \end{aligned}$$

$$F_r = (\Sigma \text{ Vertical forces}) \times f$$

Type	$W_x$	$d_x$	$M_x$
Front Wall	1450.00 lbs	1.13	1638.50
Rear Wall	1160.00 lbs	6.00	6960.00
Tie Backs	162.40 lbs	3.50	568.40
Stone	4017.60 lbs	3.50	14061.60
Footing	1015.00 lbs	----	-----
Backfill	2112.50 lbs	5.68	11999.00
	$\Sigma W_x = 9,917.50 \text{ lbs}$		$\Sigma M_x = 35,227.50$

$$\begin{aligned}
 F_r &= [P_a \sin(\delta + \beta - 90) + P_s \sin(\delta + \beta - 90) + \Sigma W_x] (f) + P_p \\
 &= [4367.19 (.22) + 0 + 9917.50] (.60) + 1006.2 \\
 &= 7533.17
 \end{aligned}$$

$$FS_s = \frac{F_r}{F_s} = \frac{7533.17}{4236.17} = 1.8 \geq 1.5 \text{ O.K.}$$

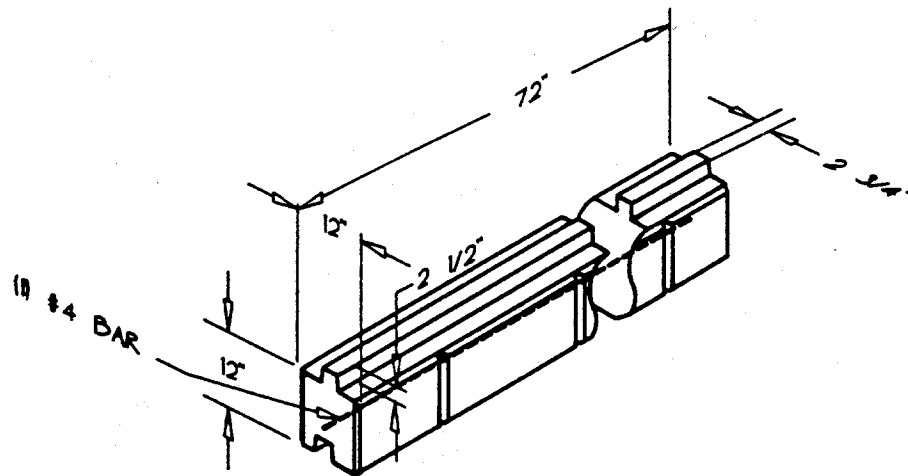
### 3. Determine stability against overturning about the toe:

a. Find  $M_o$ ,  $M_r$  and  $FS_o$ :

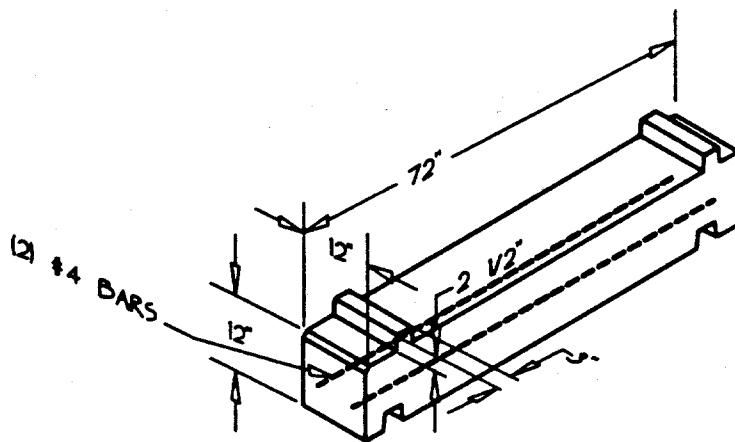
$$\begin{aligned}
 M_o &= [P_a \cos(\delta + \beta - 90)] (H_a/3) + [P_s \cos(\delta + \beta - 90)] (H_a/2) \\
 &= 4367.19 (.97) (4.17) + 0 \\
 &= 17,664.85 \text{ ft. lbs.}
 \end{aligned}$$

$$\begin{aligned}
 M_r &= (W_x) (d_x) + [P_a \sin(\delta + \beta - 90)] (d_a) + P_p (d_p) \\
 &= 35227.5 + 4367.19 (.22) 6.52 + 1006.22 \\
 &= 42498.02 \text{ ft. lbs.}
 \end{aligned}$$

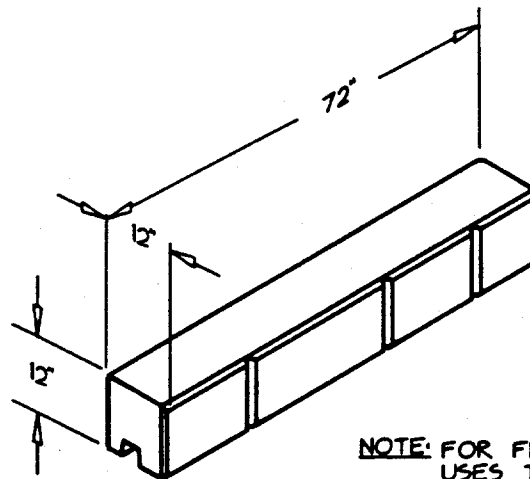
$$FS_o = \frac{M_r}{M_o} = \frac{42498.02}{17664.85} = 2.4 \geq 2.0 \text{ O.K.}$$



**STANDARD UNIT**



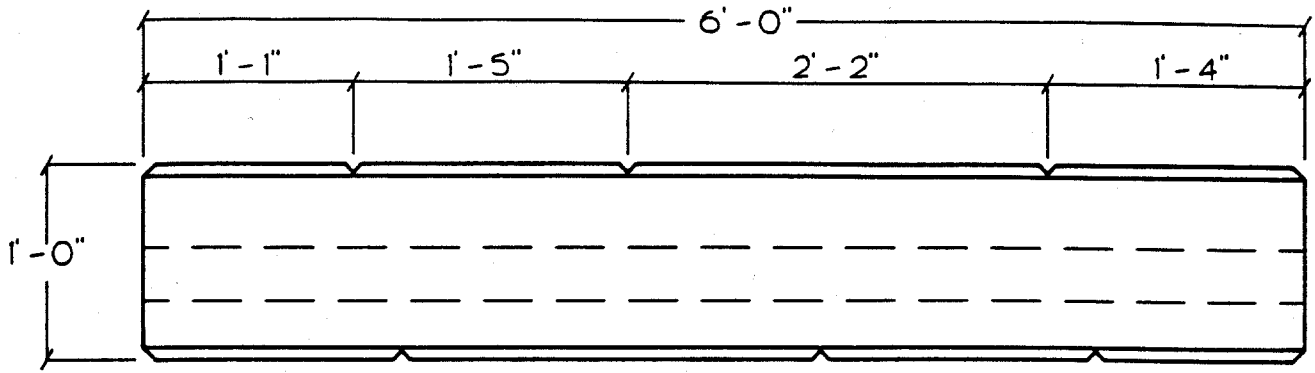
**TIE-BACK UNIT**



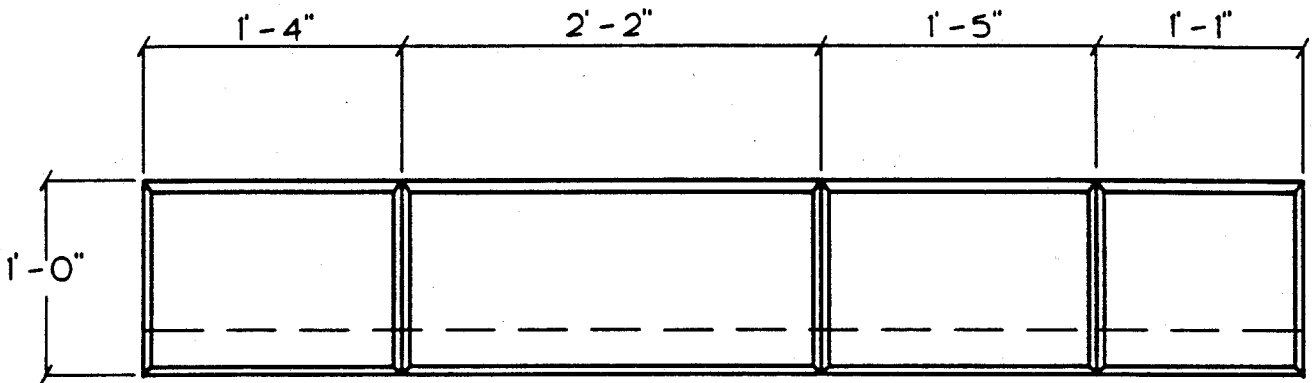
**COPING UNIT**

**NOTE:** FOR FLEXIBILITY IN CONSTRUCTION USES THE SIZE, LOCATION AND NUMBER OF REINFORCING BARS MAY BE DESIGNED BASED UPON SPECIFIC PROJECT REQUIREMENTS.

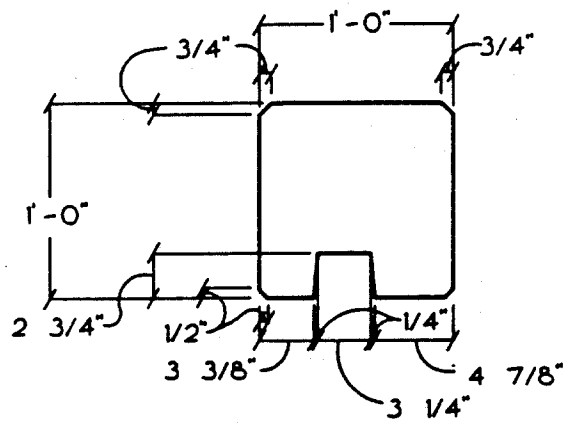
**SHOP DRAWING FOR DURA-HOLD II  
TYPICAL UNITS**



**PLAN**

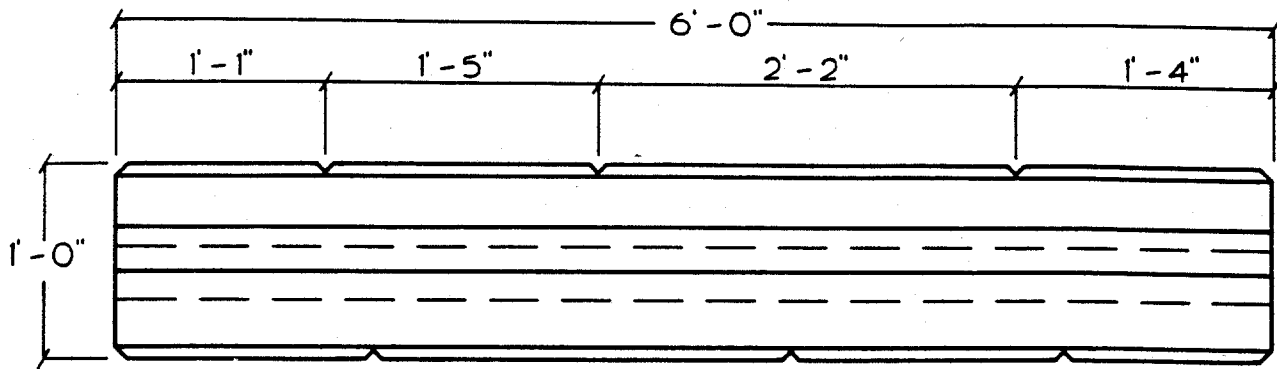


**ELEVATION**

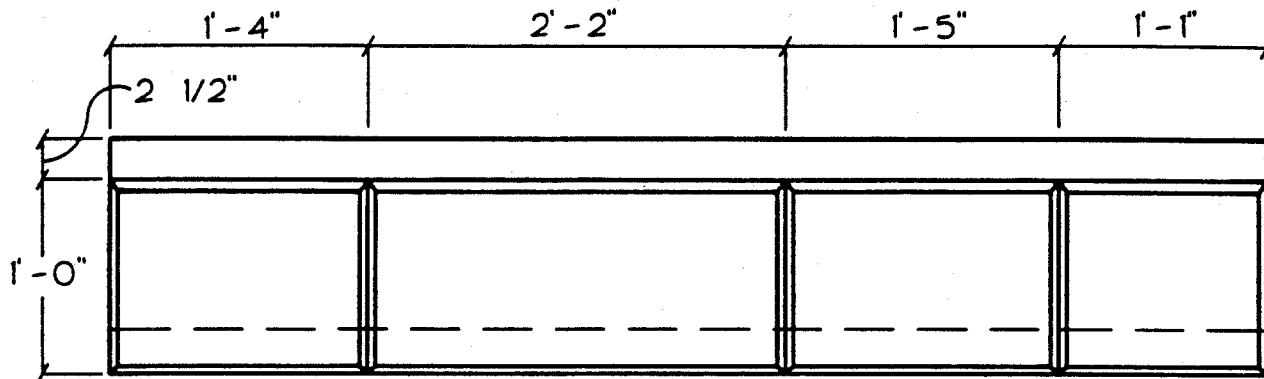


**SECTION**

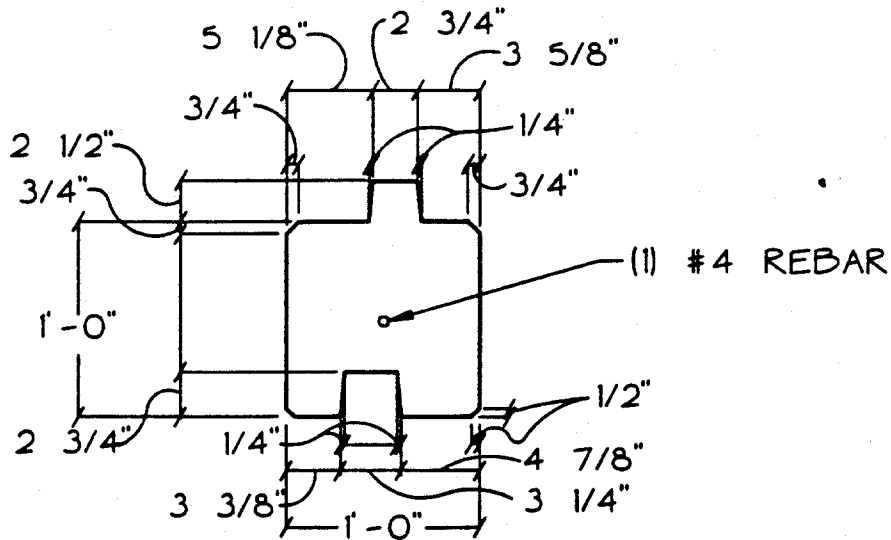
**SHOP DRAWING FOR DURA-HOLD II  
TYPICAL COPING UNIT**



**PLAN**



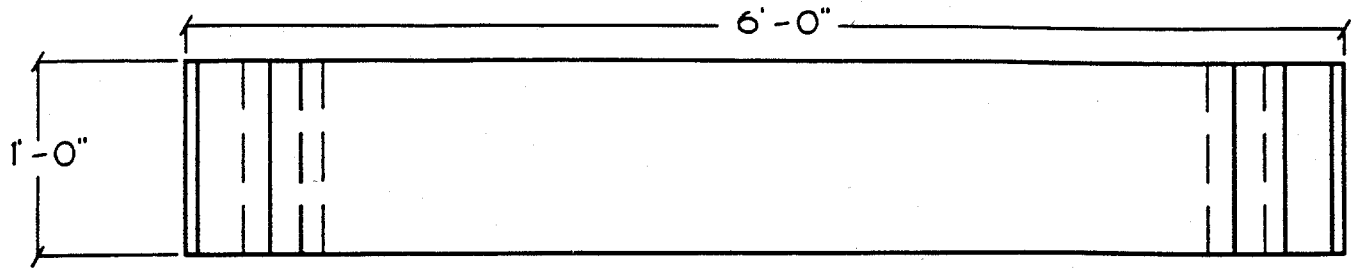
**ELEVATION**



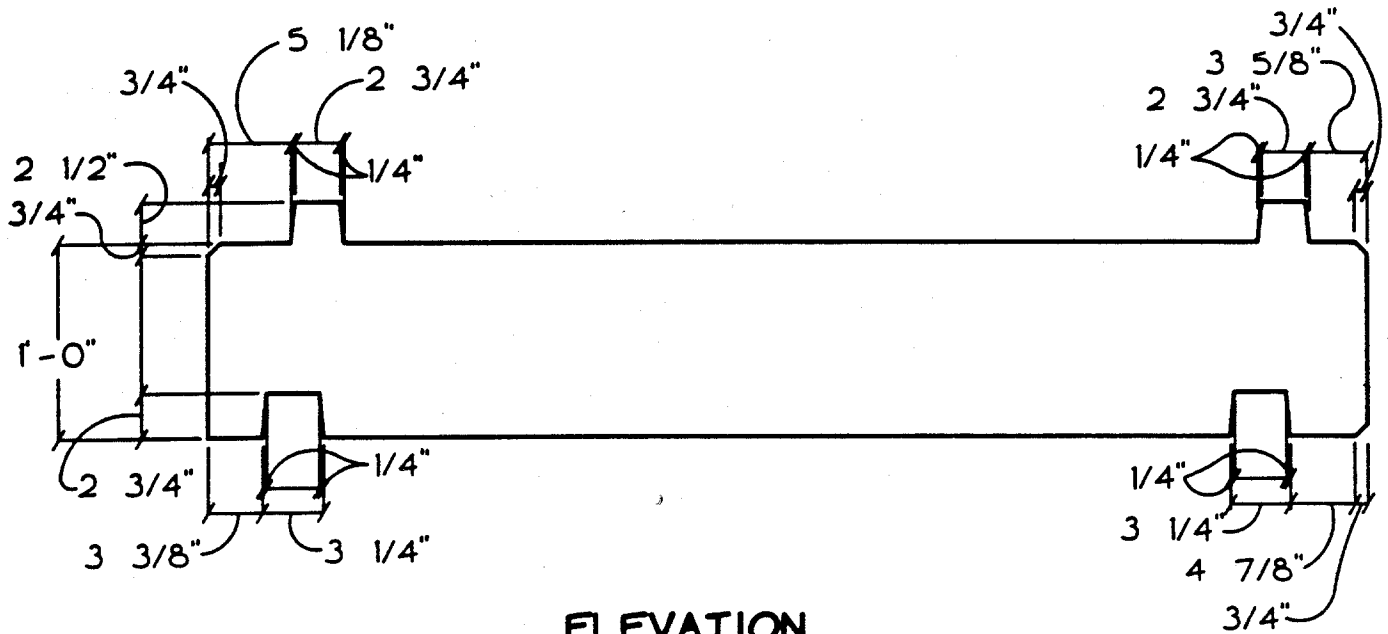
**SECTION**

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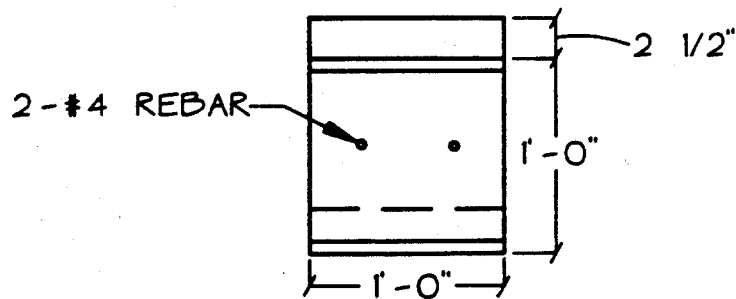
**SHOP DRAWING FOR DURA-HOLD II  
TYPICAL STANDARD UNIT**



**PLAN**



**ELEVATION**



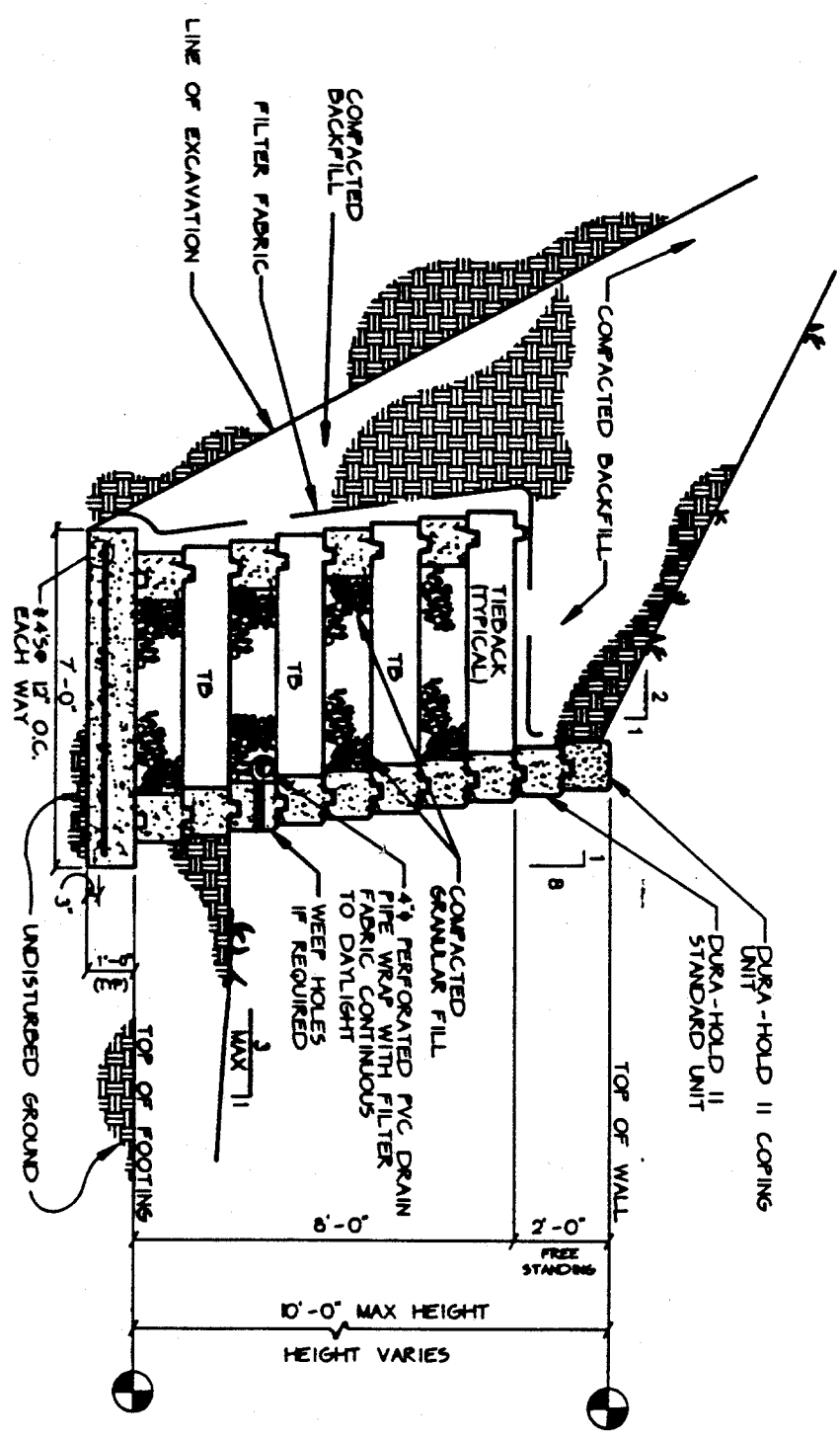
**SECTION**

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**SHOP DRAWING FOR DURA-HOLD II  
TYPICAL TIEBACK UNIT**

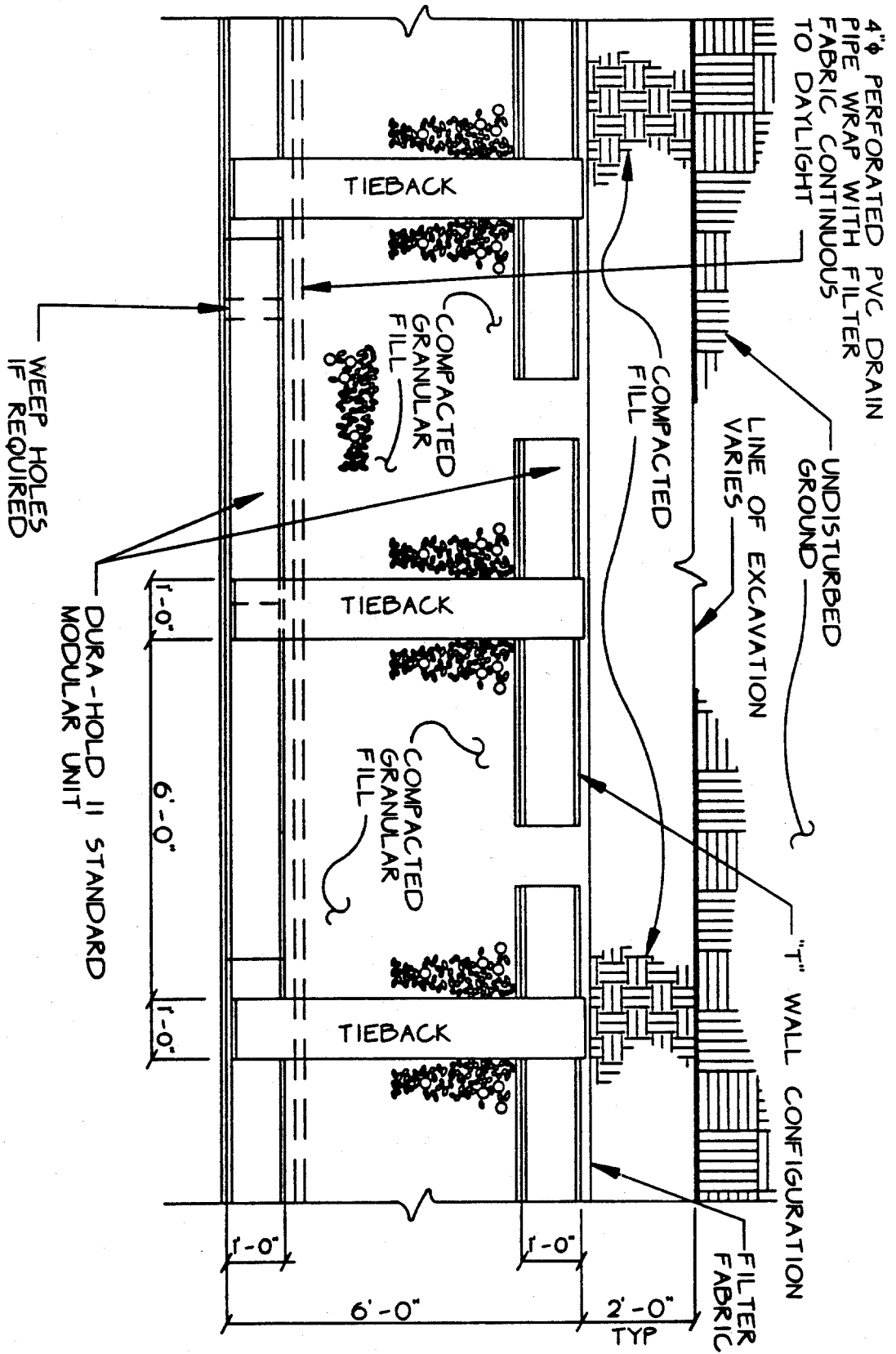
NOTE DEPTH OF EMBEDMENT, FOOTING TYPE, SIZE AND REINFORCING DEPENDS UPON CONDITIONS.

**WALL SECTION**  
SCALE: 1/4" = 1'-0"



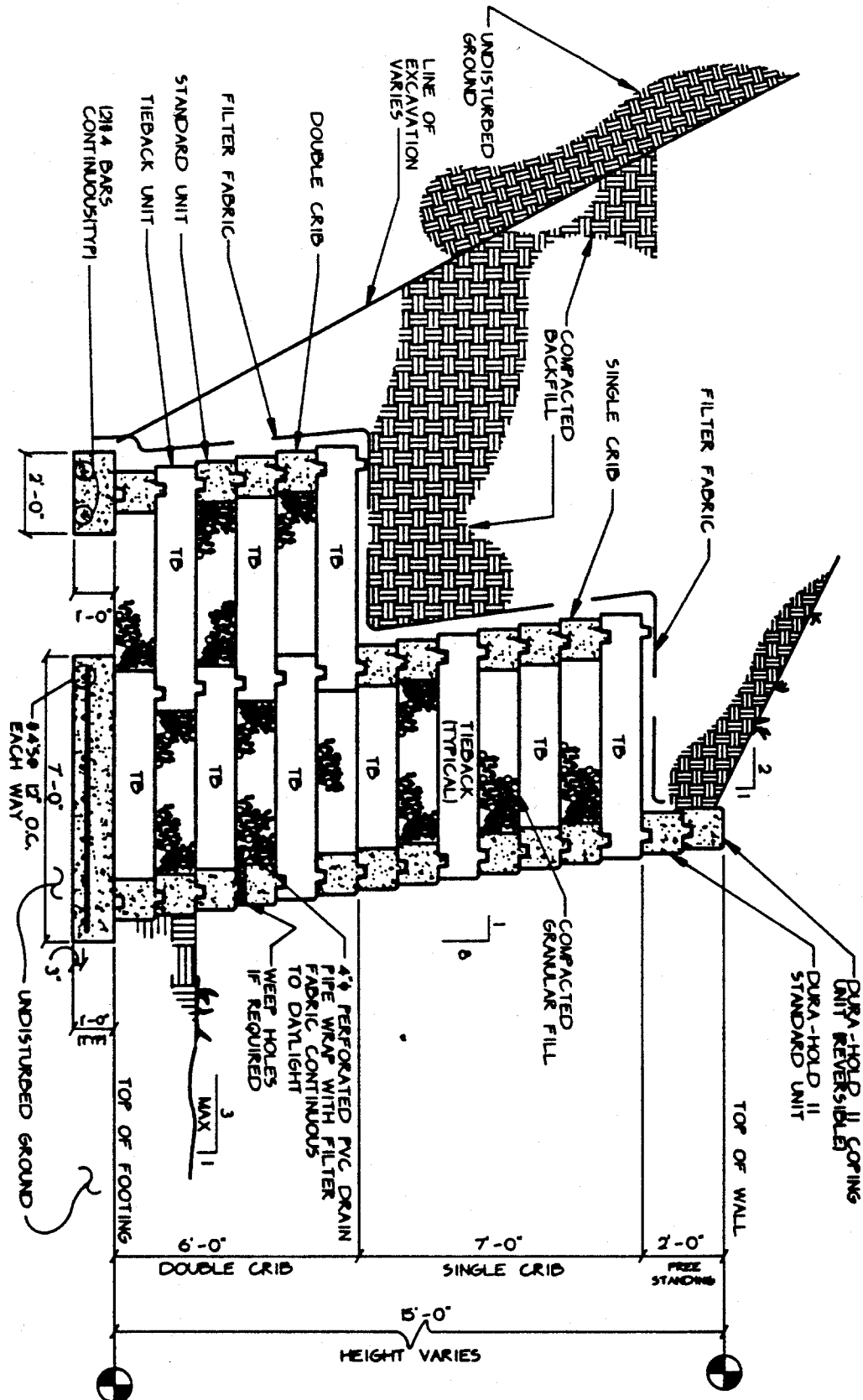
**SHOP DRAWING FOR TYPICAL  
DURA-HOLD II SINGLE CRIB WALL**





**SINGLE CRIB "T" WALL" PLAN**  
 SCALE: 3/8"=1'-0"

**SHOP DRAWING FOR DURA-HOLD II  
 SINGLE CRIB "T" WALL PLAN**

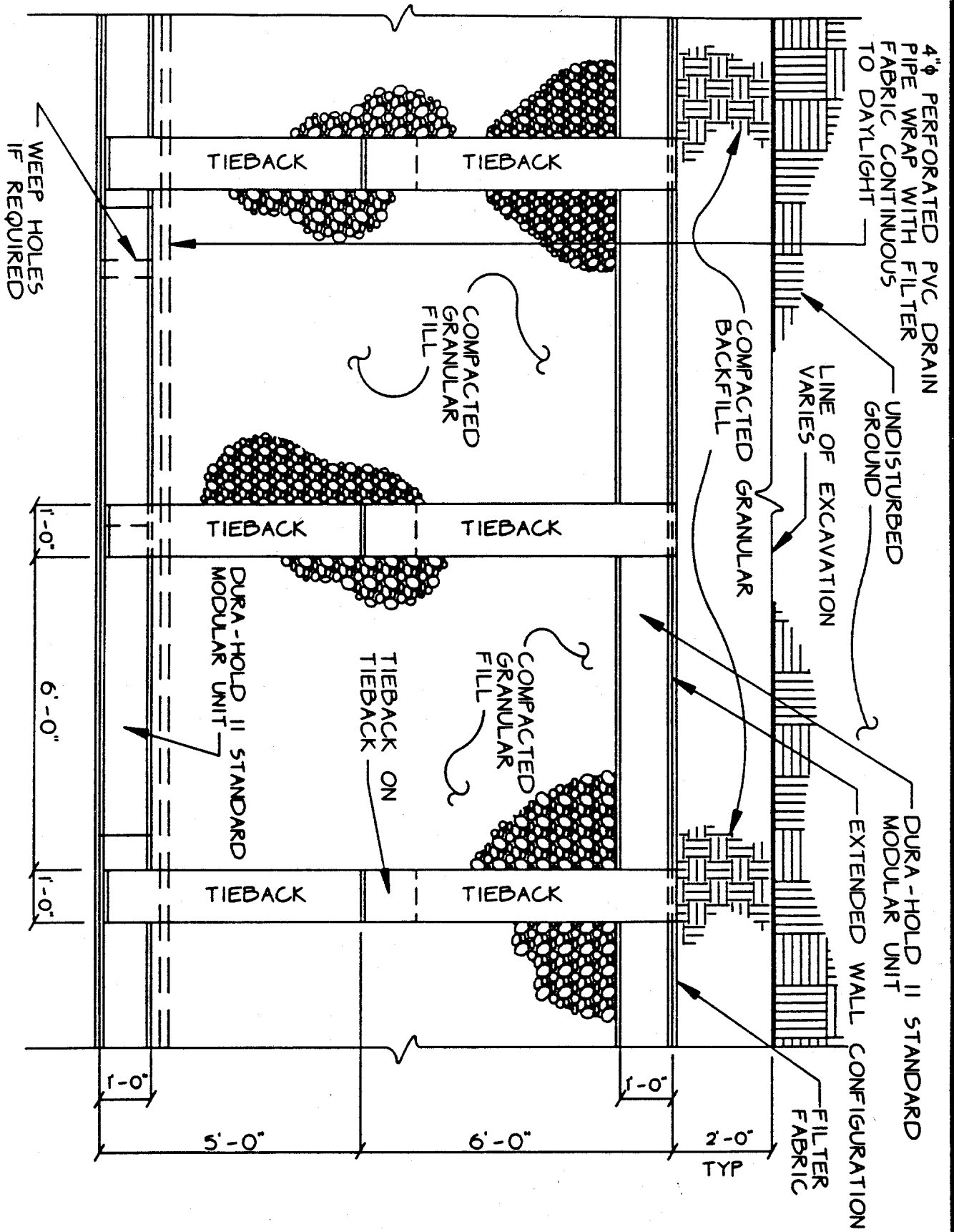


**SECTION**  
SCALE: 1/4"=1'-0"

NOTE DEPTH OF EMBEDMENT, FOOTING TYPE, SIZE AND REINFORCING DEPENDS UPON CONDITIONS.

**SHOP DRAWING FOR TYPICAL DURA-HOLD II DOUBLE CRIB WALL**

**DOUBLE CRIB TIEBACK ON TIEBACK EXTENDED WALL PLAN**  
 SCALE: 3/8"=1'-0"



**SHOP DRAWING FOR TYPICAL DURA-HOLD II DOUBLE CRIB TIEBACK ON TIEBACK EXTENDED WALL PLAN**